(ii) Dead and wind loads

• Windward side

dead + wind = $0.9 G_k + 1.4 W_k$

stress = $(0.9 \times 1.26)/1.4 - 1.4 \times 0.498$ (from Table 12.3);

(proportionately reduced from above)

$$= 0.81 - 0.70 = 0.11 \text{ N/mm}^2$$
 (no tension develops)

• Leeward side

 $dead + wind = 1.4 G_k + 1.4 W_k$

 $stress = 1.26 + 1.4 \times 0.498 = 1.96 \, N/mm^2$

(iii) Dead, imposed and wind loads

dead + imposed + wind =
$$1.2 G_k + 1.2 Q_k + 1.2 W_k$$

stress = $(1.26 \times 1.2)/1.4 + (0.13 \times 1.2)/1.6 \pm 1.2 \times 0.498$
= $1.08 + 0.098 \pm 0.98$
= 1.78 or 0.58 N/mm^2 (no tension develops)

The worst combination for this wall just above ground level also is dead+wind, and the design load is $(1.96 \times 102.5 \times 10^3)/10^3 = 201$ kN/m.

12.6.4 Selection of brick and mortar for inner leaf of wall B

The design vertical load resistance of the wall is $(\beta t f_k) / \gamma_m$ (clause 32.2.1). The value of β depends on the eccentricity of loading; hence the value of *e* needs to be evaluated before design can be completed.

12.6.5 Calculation of eccentricity

The worst combination of loading for obtaining the value of e at top of the wall is shown in Fig. 12.6. Axial load

$$P$$
=(0.9×78.54+1.6×7.29) (G_k and Q_k from Table 12.2)
=(70.69+11.66)=82.35kN/m

First floor load

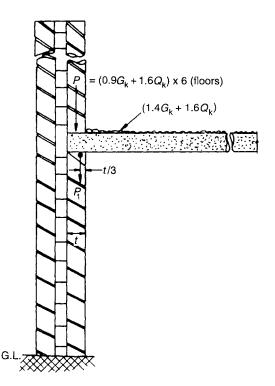


Fig. 12.6 Load combination for calculating the eccentricity.

Eccentricity

$$e = \frac{P_1 t}{6(P + P_1)} = \frac{12.31 t}{6(82.35 + 12.31)} = 0.0217t = 2.22 \text{ mm}$$

(a) Wind blowing north-south direction

A part of the panel B will be subjected to suction, if the wind is blowing in N-S direction. Then

 $V_s = VS_1S_2S_3 = 50 \times 1 \times 1 \times 0.64$ (ground roughness category A, CP3,

=32m/s

Note that the localized effect is considered here, hence S_2 for Category A is being used. Also

$$q = 0.613 \times (32)^2 = 627.8 \,\mathrm{N/mm^2}$$